

Influence of the patch loading length on the buckling coefficient of longitudinally stiffened plate girders

Influencia de la longitud de carga concentrada sobre el coeficiente de pandeo de vigas esbeltas rigidizadas longitudinalmente



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ABSTRACT: Currently, one of the most used steel bridge assembly methods is the Incremental Launching Method (ILM). Its practical application consists in passing the bridge assembly through a launching shoe as well as over each support pile. For steel plate girders, a concentrate vertical reaction also known as patch loading is generated over one the flanges when ILM is employed, and depending on the geometrical and material properties of the girder, buckling failure in the web panel may occur. To overcome this type of failure, plate girders are reinforced with longitudinal stiffeners. Therefore, this paper aims at investigating the effect of the bearing length on the elastic buckling behavior of longitudinally stiffened girder webs subjected to patch loading. Buckling coefficients of longitudinally stiffened girder webs are calculated by means of linear finite element analysis. Furthermore, a parametric analysis is performed to study the influence of other geometric parameters such as the panel aspect ratio and the geometrical properties of the longitudinal stiffener on the buckling coefficient. The results show that for longitudinally stiffened girder webs the buckling coefficient increases with the loading length. However, this conclusion is considerably affected by other factors such as the position of the stiffener, and panel aspect ratios.

RESUMEN: Uno de los métodos de ensamblaje de puentes de acero más utilizados en la actualidad es el Método de Lanzamiento por Empujes Sucesivos, también abreviado como ILM por sus siglas en inglés. Su aplicación práctica consiste en pasar el ensamblaje de puentes a través de las zapatas de lanzamiento, así como sobre cada pila de soporte. Para vigas de acero se genera una reacción vertical de carga concentrada sobre una de las aletas cuando se emplea ILM, y dependiendo de las propiedades geométricas y de material de la viga esto puede ocasionar una falla de pandeo localizado sobre el alma. Para evitar este tipo de fallas, las vigas son reforzadas con rigidizadores longitudinales. Muchas investigaciones han estudiado la influencia de diferentes parámetros geométricos sobre el comportamiento de pandeo elástico de vigas sujetas a carga concentrada, sin embargo, el efecto de la longitud de apoyo ha recibido poca atención. Este artículo se enfoca en investigar la influencia de la longitud de apoyo en el comportamiento de pandeo elástico de vigas esbeltas con rigidización longitudinal sujetas a carga concentrada. El coeficiente de pandeo de las vigas esbeltas rigidizadas longitudinalmente fue calculado por medio análisis lineal por elementos finitos. Para ello se realizó un estudio paramétrico con el propósito de evaluar la influencia de otras variables adicionales como la relación de aspecto del panel y las propiedades geométricas de los rigidizadores sobre el coeficiente de pandeo. Los resultados muestran que para una viga rigidizada longitudinalmente el coeficiente de pandeo incrementa con la longitud de carga. Sin embargo, este incremento se ve considerablemente afectado por otros factores tales como la posición del rigidizador y la relación de aspecto del panel.

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Notation

a	length of web panel
b_f	width of flange
b_{st}	width of stiffener
b_1	position of longitudinal stiffener
D	flexural rigidity of unit width of the web plate [$= Et_w^3/12(1 - \nu^2)$]
E	Young's modulus
F_{cr}	critical buckling load
F_U	Ultimate loads
F_y	Yielding load according to the Eurocode EC3-Part. 1-5
h_w	depth of web panel
I_{st}	effective second moment of area of stiffener
k_F	buckling coefficient
k_{fl}	buckling coefficient for longitudinally stiffened plate girders
k_{sl}	contribution of a longitudinal stiffener to the buckling coefficient k_{fl}
s_s	length of patch load or stiff bearing length
t_f	flange thickness
t_{st}	stiffener thickness
t_w	web thickness
β	flange stiffness parameter [$= b_f t_f^3/h_w t_w^3$]
γ_s	relative flexural rigidity of the longitudinal stiffener [$= EI_{st}/Dh_v$]
γ^t	transition rigidity
ν	Poisson's ratio

1. Introduction

A proper determination of the patch loading resistance of steel plate girders is a typical structural verification that requires attention in steel bridges erected by launching. Occasionally, slender I-girders with considerable web height are used as structural alternatives. These thin-walled webs are frequently reinforced with longitudinal stiffeners to increase bending and shear strengths, and for bridge erected by incremental launching these stiffeners also provide an enhancement for patch

loading resistance. Patch loading may generate a localized failure in the web plates in the form of an intertwined mechanism of yielding and buckling often referred to as web folding or web crippling. For the sake of predicting ultimate load capacities of steel plate girders subjected to concentrated loading, researchers have separated the theoretical studies for elastic buckling load F_{cr} and for the ultimate load F_U . Ultimate load capacities can be defined by calibrating instability-related resistance functions $\chi = f(\lambda)$, where the slenderness parameter $\lambda = f(F_{cr}, F_y)$ or by using closed form solutions F_U as upper bounds. The former verification procedure is implemented in European provisions [1] whereas the latter is implemented in American provisions [2].

Several studies in the literature have been devoted to investigate cases where the load is fractionally applied over the girder flanges. In particular, for the calculation of F_{cr} , the elastic buckling analysis of both unstiffened and longitudinally stiffened girder webs is a theoretically complex problem due to the large number and range of geometrical parameters involved. Initially, studies on the linear buckling behavior of simply supported plates under compressive loading have been conducted [3–5]. It has been demonstrated that the critical buckling load of simply supported plates increases with the use of longitudinal stiffeners placed adequately [6, 7]. Additionally, results using the folded plate theory have shown that a stiffener placed at one-fifth of the girder depth increases considerably the critical load for patch loading [8]. The contribution of flanges on the buckling coefficients for patch loading was also investigated [9].

Later on, an extensive numerical investigation led to the inclusion of the positive effect of longitudinal stiffening into the formulation for buckling coefficients of longitudinally stiffened webs under patch loads [10]. Moreover, numerical analyses have been performed to investigate the rotational restraints provided to the web plates by the flanges in unstiffened webs, developing new formulae for crane girders [11]. A linear buckling analysis was carried out to describe the behavior of unstiffened plates under interacting patch loading and bending moment [12]. Recently, a series of numerical studies were conducted to investigate the elastic buckling of unstiffened and stiffened panels subjected to opposite patch loading for different boundary conditions [13]. Furthermore, factorial design analysis was also employed to examine the influence of several geometric parameters on the buckling coefficient of longitudinally stiffened girder webs [14].

The results from the studies mentioned above can be summarized as follows, the influence of several geometric parameters has been investigated such as: (a) panel

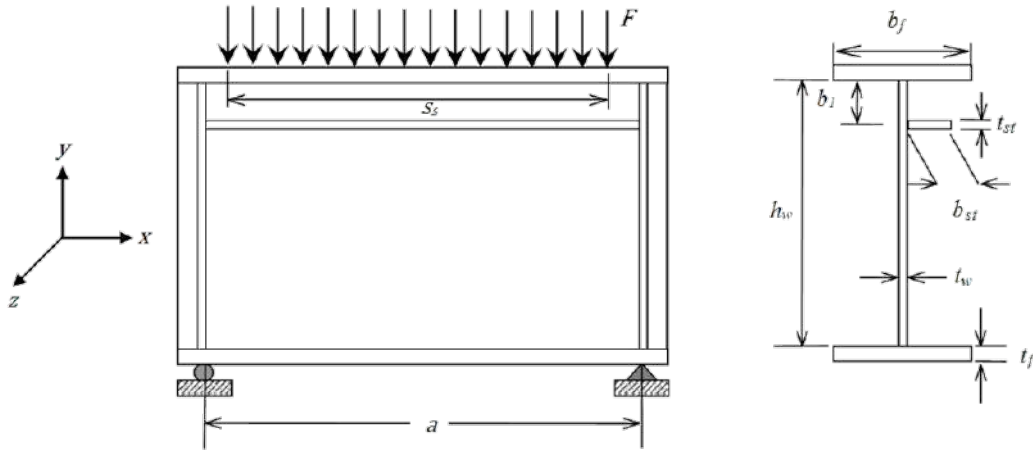


Figure 1 Longitudinally stiffened plate girder subjected to continuous loading (see notation)

aspect ratio; (b) flange stiffness parameter; (c) size of the longitudinal stiffener; and (d) relative position of the stiffener. However, there are still some uncovered issues as for instance, in the patch loading case the available formulae were developed for short loading lengths [15–17]. Some works related to this influence have been presented for ultimate load capacities rather than for elastic critical buckling loads [18–24]. Therefore, this paper aims at investigating the influence of the loading length on the buckling coefficients and transition rigidity for longitudinally stiffened girders subjected to patch loading. Buckling coefficients are computed numerically by means of linear finite element analysis; the patch loading length is varied to cover cases with large loading. These analyses are also conducted for various panel aspect ratios, stiffener positions, and stiffener rigidities. The results show that the buckling coefficient increases with increasing load lengths for certain aspect ratios. Aspect ratios and stiffener rigidities shape these observations to a major extent as described in the following.

2. Numerical model

Figure 1 shows the nomenclature used herein for a longitudinally stiffened girder subjected to patch loading. According to the classical buckling theory [25–27], the critical buckling load F_{cr} of a plate is expressed as displayed in Equation (1):

$$F_{cr} = k_F \frac{\pi^2 E t_w^3}{12(1 - \nu^2) h_w} \quad (1)$$

where k_F is the buckling coefficient that depends on edge support conditions, on stress type and on the panel aspect ratio of the plate element.

For simplicity, a parameter commonly used to describe the capacity of a stiffener to resist buckling is the flexural rigidity (γ_s) of the stiffener, which can be expressed relative to the plate stiffness as shown in Equation (2)

$$\gamma_s = \frac{EI_{st}}{h_w D} \quad (2)$$

where $D = Et_w^3/12(1 - \nu^2)$ is the flexural rigidity of a unit width of the web plate. The second moment of area I_{st} of the longitudinal stiffener is calculated with respect to its centroidal axis parallel to the web plate, for an effective cross section consisting of the stiffener itself and an effective portion of the web plate having a width of $15t_w$ on each side of the stiffener weld [1].

Buckling coefficients are computed herein through eigenvalue analysis using the FE-program ANSYS [28]. Shell elements *S181* from the ANSYS element library [28] were used to model the web, flanges (top and bottom) and the longitudinal stiffener. Due to the symmetry in geometry, loading and boundary conditions only one-half of the girder was modeled. Transverse stiffeners at the end of the girder were replaced with kinematic constraints, i.e., displacements in y and z directions, and rotation in x direction were restricted. To model the symmetry, displacement in the x direction, and rotation in y and z directions were restrained (Figure 1). A patch load was applied by loading all the nodes located in the loaded flange along a loading length equal to $s_s/2$, displacements in the x and z directions, and all rotations were restricted, allowing only vertical displacement in the y direction (following the hypothesis of a stiff bearing length s_s).

For simplicity, a rectangular cross section of the stiffener (flat stiffener) is used in the parametric analysis. The

following geometry is used as a basis for the analysis: $h_w = 1000$ mm, $a = 1000$ mm, $t_w = 4$ mm, $t_f = 8$ mm, $b_f = 250$ mm, $b_1 = 200$ mm, $s_s = 200$ mm, $t_{st} = 3$ mm. Figure 2 shows the adopted finite element mesh with 4872 elements after performing a convergence study (see Figure 3). Additionally, the buckling coefficients are calculated using a Young's modulus $E = 210$ GPa and Poisson's ratio $\nu = 0.3$. Table 1 shows a comparison between buckling coefficients computed with the numerical model and those obtained numerically in [10]. It is worth noticing that the buckling coefficients in Table 1 were obtained for a fixed loading length ($s_s/h_w = 0.2$). A very good agreement is observed for a panel aspect ratio $a/h_w = 1.0$ with greater but acceptable differences for $a/h_w = 2.0$ and $a/h_w = 4.0$.

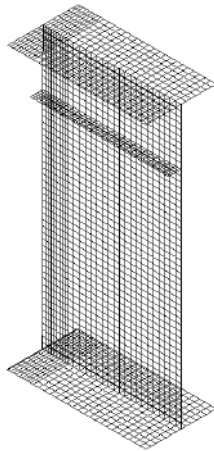


Figure 2 Finite element mesh of a longitudinal stiffened plate girder

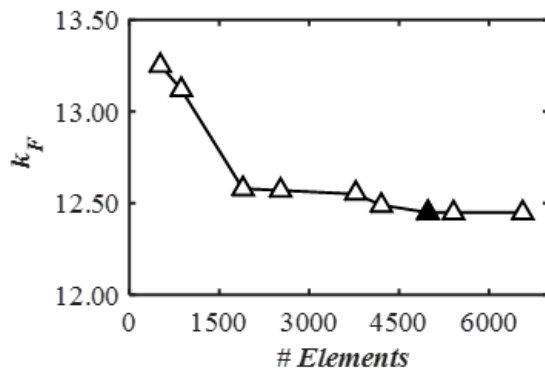


Figure 3 Convergence of the finite element mesh

3. Parametric study

In the literature, several investigations [10, 11, 13, 14] on buckling coefficients for longitudinally stiffened girder

Table 1 Comparison of k_F computed using FEM with those obtained in [10]

a/h_w	γ_s	k_F	$k_F[10]$	$k_F/k_F[10]$
1	0	8.61	8.50	1.01
	14.8	12.45	12.43	1.00
	40.4	12.98	12.95	1.00
	86.2	13.23	13.22	1.00
	158.1	13.50	13.49	1.00
	406.6	14.23	14.19	1.00
2	0	6.76	6.80	0.99
	14.8	7.43	7.44	1.00
	40.4	8.42	8.36	1.01
	86.2	9.71	9.52	1.02
	158.1	10.80	10.42	1.04
	406.6	12.07	11.38	1.06
4	0	6.96	6.59	1.06
	14.8	6.90	6.87	1.00
	40.4	7.18	7.09	1.01
	86.2	7.52	7.34	1.02
	158.1	8.01	7.64	1.05
	406.6	9.43	8.56	1.10

webs have focused their attention on the influence of the panel aspect ratio (a/h_w), relative position of the stiffener (b_1/h_w), relative flexural rigidity of the stiffener (γ_s), and the load length-to-panel height ratio (s_s/h_w). Nevertheless, in most cases the girder height (h_w) has been kept constant while the panel width is increased (a), which leads to the consideration of short loading lengths. Therefore, in order to investigate the influence of the loading length as a more refined parameter, the stiff bearing length-to-panel width ratio (s_s/a) was used herein.

In this section, a parametric analysis is conducted in order to evaluate the influence of the following parameters:

- Patch loading length was varied s_s/a : [0.1, 0.2, 0.3, 0.4, 0.5, 0.6].
- Relative position of the stiffener b_1/h_w = [0.1, 0.2, 0.3].
- Relative flexural rigidity of the stiffener γ_s = [0, 14.8, 40.4, 86.2, 158.1, 406.6], corresponding to six values of stiffeners thickness t_{st} = [0, 3, 4, 5, 6, 8] mm.
- Panel aspect ratio a/h_w = [1, 2, 4].

It is worth mentioning that for the sake of minimizing the influence of the torsional rigidity of the longitudinal stiffener in the studied phenomenon, the width-to-thickness ratio was fixed to $b_{st}/t_{st} = 15$.

Figures 3 to 5 present the buckling coefficients computed for various panel aspect ratios a/h_w and loading lengths s_s/a , the relationship between the buckling coefficients and the relative flexural rigidity of the stiffeners may be clearly observed. Moreover, the buckling coefficient of longitudinally stiffened girder webs subjected to a fixed patch loading length ($s_s/h_w = 0.2$) presented in [10], is calculated using the following expressions:

$$k_F = 5.82 + 2.1 \left(\frac{h_w}{a} \right)^2 + 0.46 \sqrt[4]{\frac{b_f t_f^3}{h_w t_w^3}} + k_{sl} \quad (3)$$

$$k_{sl} = \left(5.44 \frac{b_1}{a} - 0.21 \right) \sqrt{\gamma_s} \quad (4)$$

$$\gamma_s \leq \gamma^t = 14 \left(\frac{a}{h_w} \right)^{2.9} + 211 \left(0.3 - \frac{b_1}{a} \right) \quad (5)$$

As can be noticed in Equations (3) to (5), the contribution of the stiffener to the critical load depends on the parameter ratio b_1/a , valid within the range of $0.039 \leq b_1/a \leq 0.3$ [29]. In the particular case of $a/h_w = 4$ and $b_1/h_w = 0.1$ ($b_1/a = 0.025$) the contribution of the stiffener [k_{sl}] to the critical load was not considered.

4. Results

4.1 Influence of the bearing length s_s/a

Figures 4 to 6 show the results for the buckling coefficients k_F in terms of the relative flexural rigidity of the stiffener γ_s for various loading lengths s_s/a , and the stiffener positions $b_1/h_w = 0.1, 0.2$ and 0.3 . It is clearly observed that the buckling coefficients k_F exhibit a bilinear behavior, where k_F varies almost linearly for small values of γ_s , until reaching a value in which the slope decreases significantly. The corresponding value of γ_s at which the slope changes is called the transition rigidity γ^t [10]. These results are similar to those obtained previously in [10, 13].

Figure 4 shows the buckling coefficient k_F computed in terms of the relative flexural rigidity γ_s for various loading lengths s_s/a , with a panel aspect ratio $a/h_w = 1$. Meanwhile, Figure 5 displays the corresponding values for $a/h_w = 2$. In both cases, the buckling coefficients increase proportionally with the relative position of the stiffener for aspect ratios $a/h_w = 1$ and 2 , and a maximum value for the buckling coefficient is achieved when stiffener is placed at $b_1/h_w = 0.3$. Conversely, Figure 6 shows the results for $a/h_w = 4$ in which however, the buckling coefficient remains rather constant with respect to the stiffener position. In all cases, the buckling coefficients increase with the bearing length.

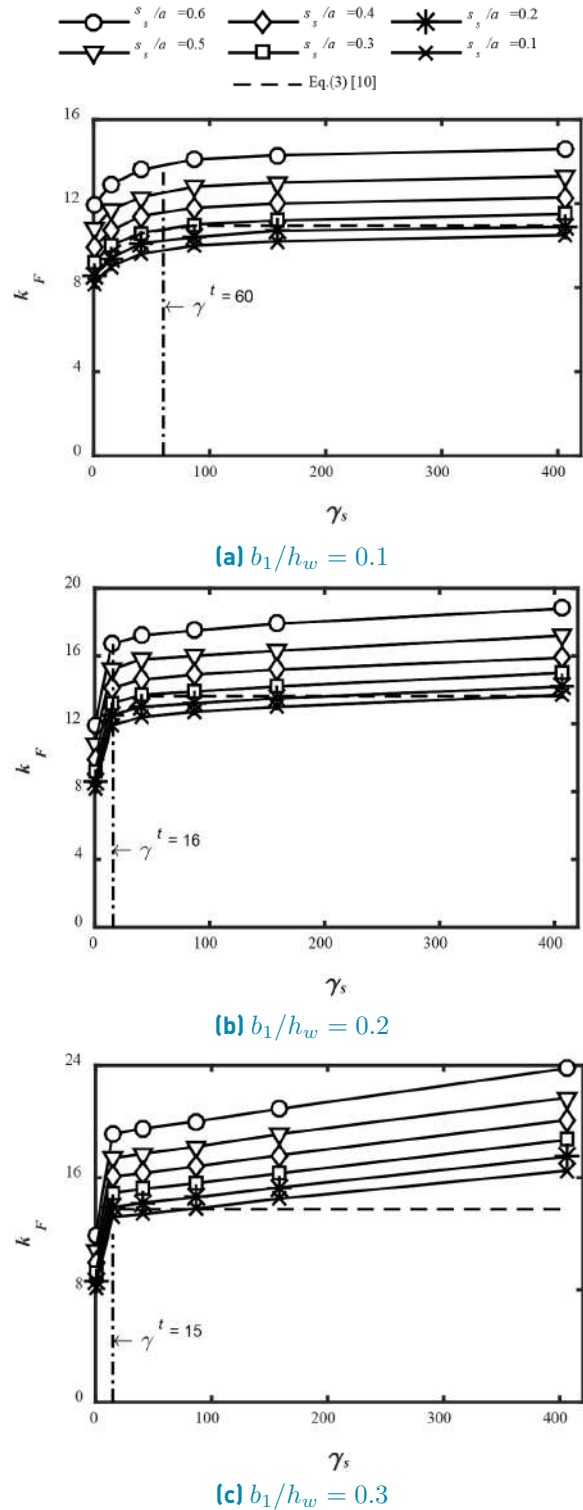
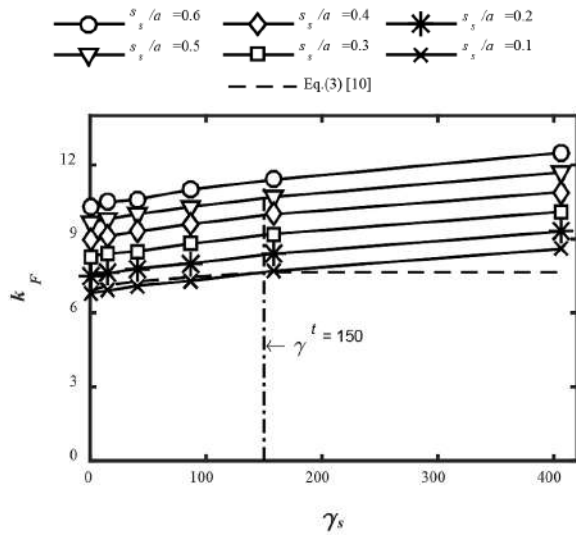
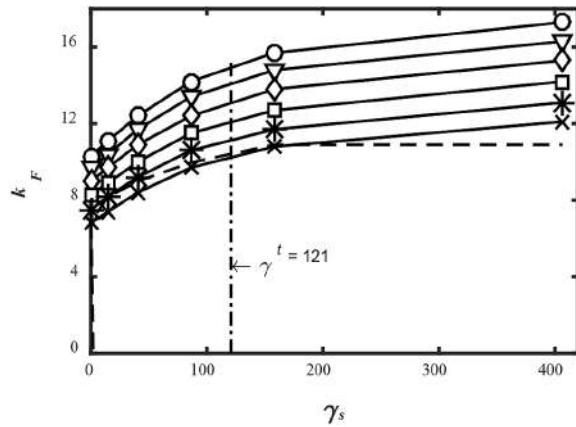


Figure 4 Buckling coefficient k_F versus the flexural rigidity of the stiffener γ_s ($a/h_w = 1$)

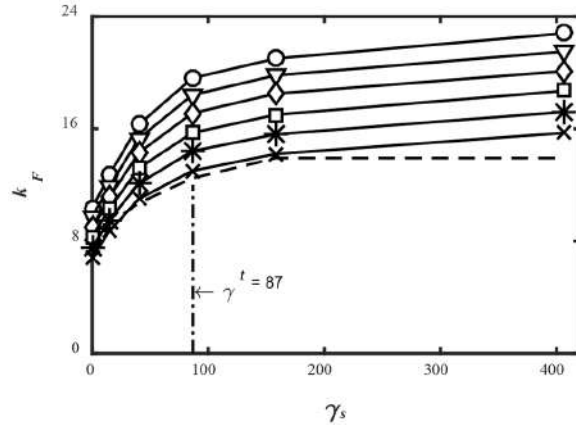
It is worth noticing that for small panel aspect ratio ($a/h_w = 1$) and a relative position $b_1/h_w \leq 0.2$, k_F values obtained with Equation (3), and based on the



(a) $b_1/h_w = 0.1$

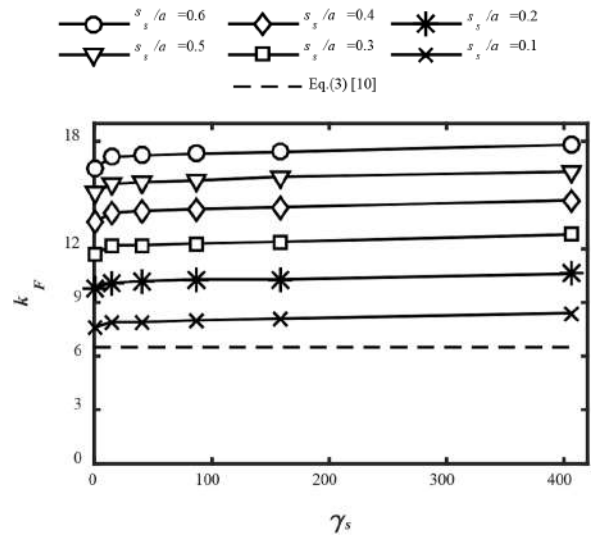


(b) $b_1/h_w = 0.2$

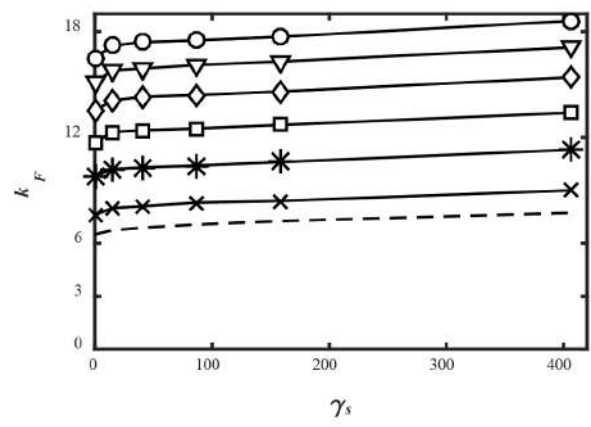


(c) $b_1/h_w = 0.3$

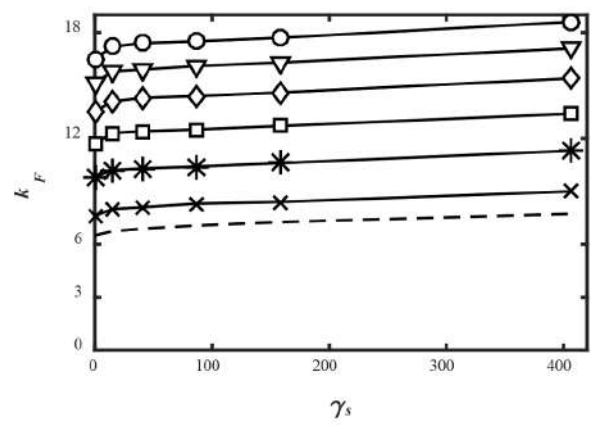
Figure 5 Buckling coefficient k_F versus the flexural rigidity of the stiffener γ_s ($a/h_w = 2$)



(a) $b_1/h_w = 0.1$



(b) $b_1/h_w = 0.2$



(c) $b_1/h_w = 0.3$

Figure 6 Buckling coefficient k_F versus the flexural rigidity of the stiffener γ_s ($a/h_w = 4$)

relative flexural rigidity restriction given in Equation (5), show a close agreement with numerical values for small patch loading length (from $s_s/a = 0.1$ to 0.2). However,

for medium or large panel aspect ratios ($a/h_w = 2$ and 4) the buckling coefficients obtained with Equation (3) are lower compared to those computed numerically herein.

In addition to the influence of the position of the stiffener, the variation of the buckling coefficient is also related to the load length s_s . Figure 7 displays values of k_F in terms of s_s/a for various relative flexural rigidities $\gamma_s = 14.8, 80.6$ and 158.1 at which the influence of the aspect ratio is also studied. For the $a/h_w = 1$ (see Figures 7a to 7c) there is a nonlinear increment of buckling coefficients as s_s/a is increased, while for the other cases $a/h_w = 2$ and $a/h_w = 4$ the increment is rather linear (Figures 7d to 7i).

At the same time, another noticeable effect is present when the aspect ratio is $a/h_w = 4$, Figures 7g to 7i show that for any particular value of s_s/a , there is no influence of the relative position of the longitudinal stiffener on the buckling coefficient (these elements experience global buckling).

4.2 Transition rigidities

As mentioned above, transition rigidities γ^t provide a limit in the k_F versus γ_s curves. The values of the transition rigidities are calculated by intersecting two straight lines obtained through linear regression. A first straight line that captures the rapid increases of k_F with respect to γ_s and a second one that describe the slightly increase of k_F vs. γ_s . With, γ^t determined as the intersection point of the two straight lines. Table 2 shows the results of transition rigidity γ^t for each calculation; these values remain constant for a patch loading length s_s/a . For panel aspect ratios $a/h_w = 1$ and $a/h_w = 2$ the transition rigidity γ^t decreases for an increasing stiffener position b_1/h_w . This means that according to the position, the transition rigidity of the stiffener varies. In both cases, the variation of γ^t with b_1/h_w follows a different trend. For $a/h_w = 1$ the results can be compared with those found in [10] where the reduction of γ^t tends to decrease to a value equal to $\gamma^t = 15$ for a relative position of $b_1/h_w = 0.3$. In the case of $a/h_w = 2$ the transition rigidity decrease linearly, nevertheless a difference of 37% with the Equation 5 is found when the stiffener is placed at $b_1/h_w = 0.3$. On the other hand, for $a/h_w = 4$ the results are similar to those obtained in [10], where the transition rigidity γ^t is higher than the relative flexural rigidity γ_s within the evaluated range.

4.3 Buckling shapes

For the sake of visual inspection, Figures 8 to Figure 10 display isometric views of the buckled shapes for three selected conditions for each case ($a/h_w = 1, 2$ and 4 , respectively). Three buckled shapes were obtained for a stiffener placed at $b_1/h_w = 0.2$, with $\gamma_s = 158.1$. For panel aspect ratios $a/h_w = 1$ and 2 , this rigidity is larger than the corresponding transition rigidity ($\gamma_s > \gamma^t$). A closer

Table 2 Transition rigidities γ^t

a/h_w	b_1/h_w	s_s/a						γ^t [10]
		0.1	0.2	0.3	0.4	0.5	0.6	
1	0.1			60				56
	0.2			16				35
	0.3			15				14
2	0.1			150				157
	0.2			121				147
	0.3			87				137
4	0.1							
	0.2			>400				>400
	0.3							

inspection of the eigenmodes shows a larger wavelength in the lower panel for an increasing load length ratio s_s/a . All cases show an eigenmode that involves buckling in the lower sub-panel.

Noticeably, as shown in Figures 8 to 10, the buckling shapes are affected by the panel aspect ratio. The buckled region tends to be greater for a larger load length as well as for a wider panel. Moreover, for large panel aspect ratios $a/h_w = 4$ (see Figure 10c), there is an overall buckling mode of the web panel, in which web and longitudinal stiffener buckle together.

Summarizing, Figure 11 shows the buckling modes for the values of a/h_w and s_s/a analyzed herein. All modes are extracted in the form of the cross-sectional eigenmode at mid-span. It can be observed that the buckling mode in the center of the girder varies only with the aspect ratio instead of the load length. This variation can be observed when the aspect ratio increases as well as the deformation of the web panel. It is worth noticing that for a panel with $a/h_w = 4$, the longitudinal stiffeners do not provide a stiff nodal line.

5. Conclusions

The influence of the bearing length on the critical buckling load of girders subjected to patch loading has been analyzed numerically with a broad perspective herein. The effect of s_s/a coupled with many other parameters such as the aspect ratio of the panel, the position, and second moment of area of the stiffeners show the following conclusions:

- Generally, the critical buckling load of the plates increase with s_s/a . The effect is particularly observable in stiffened plates with low aspect ratio $a/h_w = 1$ in which the eigenmode is clearly associated with failure of the bottom panel of the directly loaded plate. In these cases, increasing s_s/a implies a considerable increment of the buckling coefficient following a nonlinear trend.

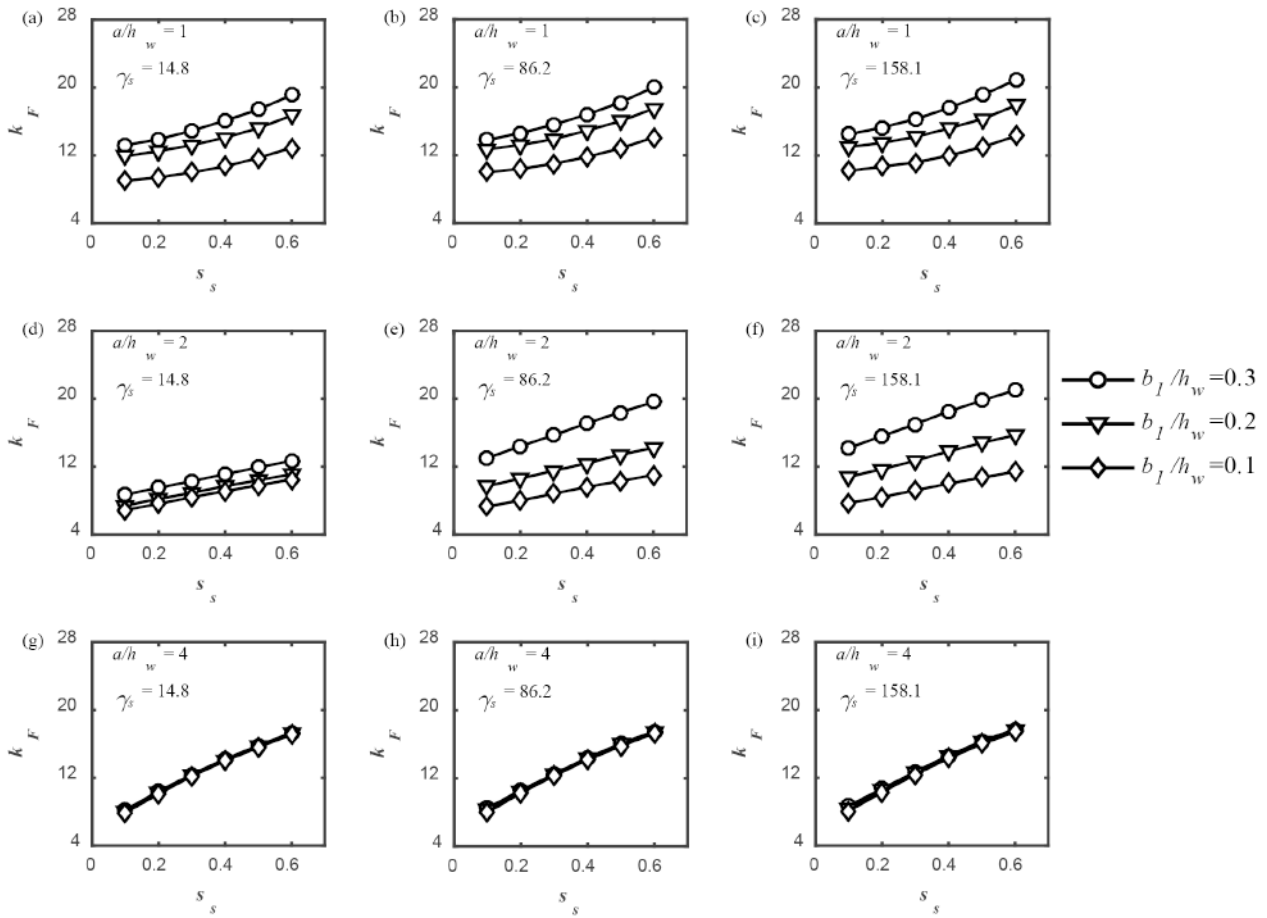


Figure 7 Buckling coefficient k_F versus the load length-to width ratio s_s/a

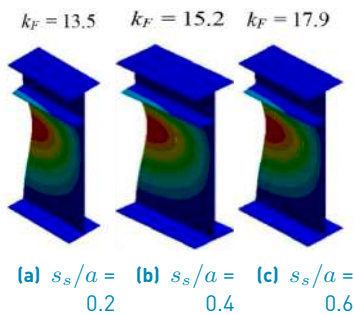


Figure 8 Buckling shapes for plate girders with various loading lengths ($a/h_w = 1$; $b_l/h_w = 0.2$; $\beta = 2$; $\gamma_s = 158.1$)

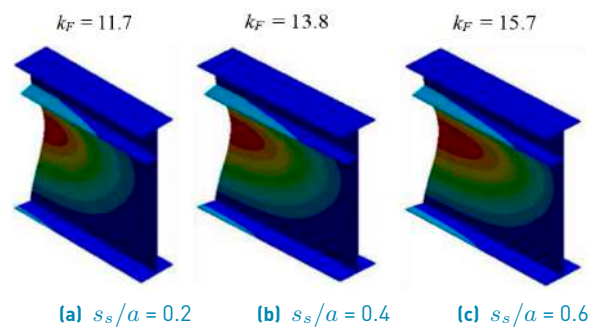


Figure 9 Buckling shapes for plate girders with various loading lengths ($a/h_w = 2$; $b_l/h_w = 0.2$; $\beta = 2$; $\gamma_s = 158.1$)

- For stiffened girders with $a/h_w = 4$, the influence of s_s/a is similar (increasing the buckling coefficient with the loading length) but in this case, the enhancement is smaller. Since the panels are considerably wide, the eigenmode partially involves the longitudinal stiffener and the transverse stiffeners do not contribute to the anchorage of such elements.

These conclusions are necessary to point out when it

comes to the design of plate girders subjected to patch loading. These elements are often designed with largely spaced transverse stiffeners and less frequently with closely spaced stiffeners. Since the launching shoes generally present standard dimensions available in the industry (1.0 to 2.0 meters), the proportion s_s/a will be determined by the binomial decision of the transverse stiffener spacing and the utilized launching equipment.

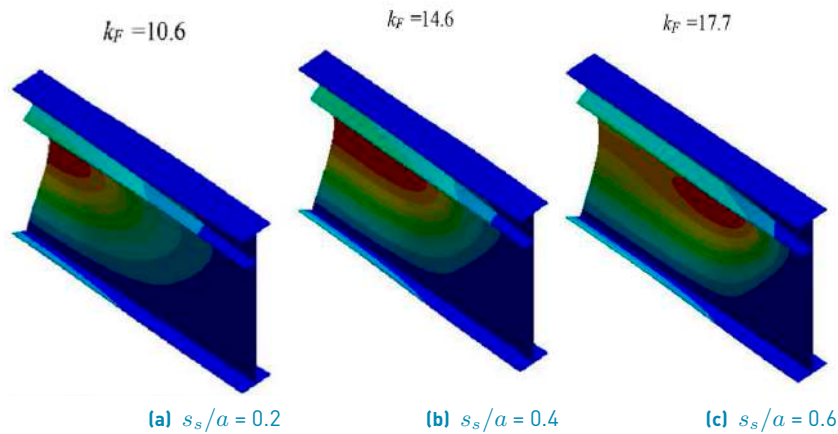


Figure 10 Buckling shapes for plate girders with various loading lengths ($a/h_w = 4$; $b_1/h_w = 0.2$; $\beta = 2$; $\gamma_s = 158.1$)










a/h_w	s_s/a		
	0.2	0.4	0.6
1	 $k_F = 13.5$	 $k_F = 15.2$	 $k_F = 17.9$
2	 $k_F = 11.7$	 $k_F = 13.8$	 $k_F = 15.7$
4	 $k_F = 10.6$	 $k_F = 14.6$	 $k_F = 17.7$

Figure 11 Buckling mode for a plate girder with patch loading lengths ($b_1/h_w = 0.2$; $\beta = 2$; $\gamma_s = 158.1$)

The resistance of webs to concentrated loads will benefit in the case of closely spaced transverse stiffeners and/or large launching shoes since coupled mechanisms involving the transverse and longitudinal reinforcements will determine the load bearing capacity of the plate girder as whole. Finally, it is recommended to study these complex coupled resistant mechanisms not only from the critical buckling load perspective but also at ultimate load capacity levels.

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